

## River Engineering

### Tutorial Sheet 1 – Resistance laws for steady uniform flow

1. Gauckler-Manning-Strickler
  - a. Plot a graph of Strickler's relationship  $k_{St} = 6.7\sqrt{g}/D^{1/6}$  for  $D$  from 0.1 mm to 30 cm (logarithmic  $D$  axis, linear  $k_{St}$  axis). Keep for future use?
  - b. Compare the two values you get for  $D = 1$  cm and 2 cm. Comment.
  - c. Do you have an explanation for the remarkable insensitivity of  $k_{St}$  to changes in boundary grain size? (If you do, please tell the lecturer. We will discuss this.).
2. For a trapezoidal cross-section, where the bottom width is  $W$ , the depth is  $h$ , and the side slopes are (H:V)  $m : 1$ ,
  - a. Show the following properties:

$$\text{Top width } B = W + 2mh$$

$$\text{Area } A = h(W + mh)$$

$$\text{Wetted perimeter } P = W + 2\sqrt{1 + m^2}h$$

- b. Verify that the general relationship  $B = dA/dh$  holds true for this type of section.
3. Use the Gauckler-Manning-Strickler formulation and the results for a trapezoidal section to obtain an expression for the steady uniform discharge as a function of depth  $h$ :

$$Q = k_{St} \frac{(h(W + mh))^{5/3}}{(W + 2h\sqrt{1 + m^2})^{2/3}} \sqrt{S_0} \quad (1)$$

4. A commonly-encountered problem in river and canal hydraulics is to compute the normal (uniform flow) depth for a given discharge by numerically solving the transcendental equation you obtained in question 3. Here we consider methods for solving the problem:
  - a. A traditional method is to plot the relationship and read the solution off. However, this is not suited to computer programs.
  - b. Trial and error – substitute numerical values of  $h$  in until the required discharge is obtained, also not suited to computer programs.
  - c. Solve the equation by a numerical method for solving transcendental equations such as direct iteration, bisection, or Newton's method, or most simply, using Solver in Excel.
  - d. The direct iteration scheme in the lectures works well:

$$h_0 = \left( \frac{Q}{k_{St} B_0 \sqrt{S}} \right)^{3/5}$$

$$h_{n+1} = \left( \frac{Q}{k_{St} \sqrt{S}} \right)^{3/5} \times \frac{P^{2/5}(h_n)}{A(h_n)/h_n} = \left( \frac{Q}{k_{St} \sqrt{S_0}} \right)^{3/5} \frac{(W + 2h_n \sqrt{1 + m^2})^{2/5}}{W + mh_n},$$

which is then evaluated repeatedly.

- e. Obtain the corresponding iteration scheme for the Chézy-Weisbach formulation.

$$h_{n+1} = \left( \frac{\lambda Q^2}{8gS_0} \right)^{1/3} \frac{P^{1/3}(h_n)}{A(h_n)/h_n} = \left( \frac{\lambda Q^2}{8gS_0} \right)^{1/3} \frac{(W + 2h_n \sqrt{1 + m^2})^{1/3}}{W + mh_n}.$$

- f. Check: is that dimensionally correct? *Always* check for dimensional correctness.
  - g. Note that such direct iteration does not work for all equations – it did work here because we made sure that our equation was a weakly varying function of  $h$ .
5. Consider a canal with  $W = 10$  m, side slopes of 2:1, a longitudinal slope of 0.0005,  $k_{St} = 50$ . For a flow of  $20 \text{ m}^3\text{s}^{-1}$ , calculate the normal depth  $h$ .
- a. If you have access to a spreadsheet with Solver installed, use that to solve the problem.
  - b. Use the direct iteration scheme in question 4.d  
(*Ans:  $h = 1.348$  m – if we start with  $B_0 = W = 10$ , we then obtain  $h_0 = 1.418$ ,  $h_1 = 1.344$ ,  $h_2 = 1.349$ ,  $h_3 = 1.348$  m, and it has converged to within a millimetre).*)